APPENDIX D

DRAINAGE REPORT WASTEWATER DISPOSAL FEASIBILITY TESTING REPORT AND 72-HOUR PUMP TEST REPORT

DRAINAGE REPORT

FOR:

Livingston's Concrete Service, Inc. Portion of APN: 040-271-042 SEC Ophir Road & Geraldson Road Ophir, CA

PREPARED BY:

OURADA ENGINEERING 3111 SUNSET BLVD., STE. L ROCKLIN, CA 95677 (916) 624-1221 Revised June 8, 2006

Drainage Report SEC Ophir Road & Geraldson Road Ophir, CA June 8, 2006

Introduction

This report was prepared to facilitate the approval of the rough grading and improving the site with a concrete batch plant. The site lies between Ophir Road on the north and Interstate 80 on the south and is just east of the intersection of Geraldson Road and Ophir Road. (Assessors Parcel Number 040-271-042). Exhibit 1 shows the site on the USGS Quad map and in relation to the nearby roadways. The developer of this property, Livingston's Concrete Service, Inc., plans to build and operate a concrete batch plant at this location. The total site is 4.9 acres and the batch plant and associated improvements will be built on the majority of the site. In the southeastern corner there will be an area that will not be graded that will be used for the septic area. Along the eastern property line and the northerly property line, large landscape areas will be provided.

Current Drainage Patterns

The current drainage is overland sheet flow from the vacant site, entering a roadside ditch. Exhibit 2 shows the pre-development flow conditions. Approximately 1.83 acres flows to the east and 2.19 acres flows to the west. Both sides ultimately cross Ophir Road in culverts and flow is then directed in swales ending in Auburn Ravine. The attached drainage calculations show a flow of 3.11 cfs for the 10-year storm and 5.49 cfs for the 100-year storm from the easterly portion. Flows of 3.29 cfs for the 10-year storm and 6.13 cfs for the 100-year storm are calculated for the westerly portion.

There is also an existing 24" storm drainpipe at the southwesterly corner of the site. This pipe appears to come from under the freeway, however, there is no scouring and/or erosion at the outlet of the pipe as would be expected for drainage coming from a pipe. However, we have calculated from the USGS Quad map (See Exhibit 1) the upland drainage area to be 11.25 acres. Per the attached calculations, this area would contribute 13.50 cfs for the 10-year event and 24.75 cfs for the 100-year event. This drainage is assumed to collect in the 24" pipe and will be directed through a lined or cobbled swale along the westerly property line. This drainage is not from the property and will not be detained and/or treated per this project. It would also be suggested to visit this site during a storm in the winter and actually measure the water coming through the pipe. Again, there was no scouring at the exit of the pipe as would be expected for this amount of drainage.

Proposed Improvements/Drainage

As mentioned above, the site (4.9 acres) will be improved with the concrete batch plant and associated parking, drive aisles, septic field and landscaping. Exhibit 3 shows the post-development conditions and the site is broken into three areas as shown. Flows from the improved easterly portion of the site (Area 3) are shown in the attached calculations which show 2.66 cfs for the 10-year storm and 4.79 cfs for the 100-year storm. Since this portion is less than the pre-development flows, this portion will not have any associated detention.

Flows from the improved westerly portion (Area 1 and Area 2) are calculated as 5.83 cfs for the 10-year storm and 10.59 cfs for the 100-year storm. All flows from the project, except from the driveway areas, will be collected on-site and flow through an on-site detention basin. Flows from the driveways will enter directly into the roadside ditches.

The on-site detention basin will be approximately 6,000 cubic feet and the calculations show that the required storage should be 2,284 cubic feet for the 100-year storm. Other storm events are not calculated as all storms will fit in this basin. This was calculated based on total flow from the site not to exceed 90% of the pre-development flows. Extra storage volume was added to keep more of the 100-year event on-site and allow the operator to use as much re-claimed water as possible for their batching operations. Water from this basin will be re-cycled and used in the batch plant operation when available. At other current Livingston Concrete sites, anywhere from 50% to 75% of the storm water is recycled and used in the batch plant operations.

Water exiting the detention basin will flow through an outlet with redwood bark will be used as a treatment for pH of the water. Livingston Concrete has demonstrated that adding water to redwood bark is an effective treatment to balance the pH of water coming from the batch plant area. An actual settling pond will be used to collect drainage and water in the actual batch plant area. All water from this pond will be recycled and used by the batch plant. Only in a heavy rain event will water ever leave this area and end up in the on-site detention area.

Erosion Control Measures and BMP's for Project

This project will comply with current State of California and Placer County erosion control practices and include tightly packed straw rolls and filter bags around the existing inlet and any new catch basins. The project will also be protected by straw rolls around the exterior, on the north side, prior to any surface flow entering the roadside ditches during construction. A SWPPP will be developed for this project since it will have a disturbed area of over 1.0 acres. As the project is processed through improvement plans, additional drainage elements may be analyzed and added as determined appropriate.

After construction, the detention and settling basin will serve as an on going BMP for storm water control and treatment. Livingston's Concrete prides itself in handling storm water issues on existing sites with state of the art practices and procedures. They will implement their years of experience for this site as well to meet all current State and Local storm water quality guidelines. There is no quantitative analysis of the redwood treatment system as this is a system that Livingston's uses on other plants that is very effective in treating the pH of the water.

Note the intention of this project is to capture as much storm water on the site as possible and use as reclaimed water in the plant.

HYDROLOGY CALCULATIONS:

ALL CALCULATIONS AND REFERENCES ARE PER THE PLACER COUNTY STORM WATER MANAGEMENT MANUAL:

PRE-DEVELOPMENT FLOWS FOR OFF-SITE 11.25 ACRES:

EXISTING CONDITIONS: POOR GRASS/MODERATELY ROUGH SURFACE, L=900' n=0.4, s=23%, ELEVATION = 1000' T(r) = 18.85 MINUTES (EQUATION 5-3)

UNIT PEAK FLOW:

T(r) = 19 MINUTES, ELEVATION = 1000'

10 YEAR FLOW q = 1.3 cfs/acre (FIG. 5-3A) 100 YEAR FLOW q = 2.3 cfs/acre (FIG. 5-3C)

INFILTRATION FACTOR:

CLASS D SOIL; OPEN SPACE/FAIR GRASS INFILTRATION RATE, I = 0.07 inches/hour (TABLE 5-3) INFILTRATION FACTOR, F(i) = 0.1 inches/hour (FIG. 5-4)

TOTAL PEAK FLOW:

DRAINAGE = 11.25 ACRES DRAINS TO NORTHWEST EQUATION 5-6: Q(p) = A(t)q - A(p)F(i)

10 YEAR FLOW Q(p) = 11.25 (1.3 - 0.1) = 13.50 cfs100 YEAR FLOW Q(p) = 11.25 (2.3 - 0.1) = 24.75 cfs

PRE-DEVELOPMENT FLOWS FOR ENTIRE SHED:

AREA A (Westerly Portion):

EXISTING CONDITIONS: POOR GRASS/MODERATELY ROUGH SURFACE, L=410' n=0.4, s=8.8%, ELEVATION = 1000' T(r) = 15.69 min. (Equation 5-3)

UNIT PEAK FLOW:

T(r) = 16 MINUTES, ELEVATION = 1000'

10 YEAR FLOW q = 1.6 cfs/acre (FIG. 5-3A) 100 YEAR FLOW q = 2.9 cfs/acre (FIG. 5-3C)

INFILTRATION FACTOR:

CLASS D SOIL; OPEN SPACE/FAIR GRASS INFILTRATION RATE, I = 0.07 inches/hour (TABLE 5-3) INFILTRATION FACTOR, F(i) = 0.1 inches/hour (FIG. 5-4)

AREA A PEAK FLOW:

DRAINAGE = 2.19 ACRES DRAINS TO NORTHWEST EQUATION 5-6: Q(p) = A(t)q - A(p)F(i)

10 YEAR FLOW Q(p) = 2.19 (1.6 - 0.1) = 3.29 cfs100 YEAR FLOW Q(p) = 2.19 (2.9 - 0.1) = 6.13 cfs

AREA B (Easterly Portion):

EXISTING CONDITIONS: POOR GRASS/MODERATELY ROUGH SURFACE,

 $L{=}370'~n{=}0.4,\,s{=}6.5\%,\,ELEVATION=1000'$

T(r) = 14.53 min. (Equation 5-3)

UNIT PEAK FLOW:

T(r) = 15 MINUTES, ELEVATION = 1000'

10 YEAR FLOW q = 1.8 cfs/acre (FIG. 5-3A) 100 YEAR FLOW q = 3.1cfs/acre (FIG. 5-3C)

INFILTRATION FACTOR:

CLASS D SOIL; OPEN SPACE/FAIR GRASS INFILTRATION RATE, I = 0.07 inches/hour (TABLE 5-3) INFILTRATION FACTOR, F(i) = 0.1 inches/hour (FIG. 5-4)

AREA B PEAK FLOW:

DRAINAGE = 1.83 ACRES DRAINS TO NORTHEAST EQUATION 5-6: Q(p) = A(t)q - A(p)F(i)

10 YEAR FLOW Q(p) = 1.83 (1.8 - 0.1) = 3.11 cfs100 YEAR FLOW Q(p) = 1.83 (3.1 - 0.1) = 5.49 cfs

POST-DEVELOPMENT FLOWS:

AREA 1: (WEST portion)

PROPOSED CONDITIONS: ASPHALT OR CONCRETE PAVING L=200' n=0.11, s=3.1%, ELEVATION = 1000' T(r) = 6.4 MIN. (EQUATION 5-3)

UNIT PEAK FLOW:

T(r) = 6.4 MINUTES, ELEVATION = 1000'

10 YEAR FLOW q = 2.4 cfs/acre (FIG. 5-3A) 100 YEAR FLOW q = 4.5 cfs/acre (FIG. 5-3C)

INFILTRATION FACTOR:

SURFACES ASSUMED IMPERVIOUS FOR THESE CALCULATIONS

PEAK FLOW:

DRAINAGE AREA = 1.28 AC TOTAL EQUATION 5-6: Q(p) = A(t)q - A(p)F(i)

10 YEAR FLOW Q(p) = 1.28 (2.4) = 3.07 cfs100 YEAR FLOW Q(p) = 1.28 (4.5) = 5.76 cfs

AREA 2: (CENTER RE-CLAIM portion)

PROPOSED CONDITIONS: ASPHALT OR CONCRETE PAVING L=225' n=0.11, s=0.5%, ELEVATION = 1000' T(r) = 11.93 MIN. (EQUATION 5-3)

UNIT PEAK FLOW:

T(r) = 12 MINUTES, ELEVATION = 1000'

10 YEAR FLOW q = 2.0 cfs/acre (FIG. 5-3A) 100 YEAR FLOW q = 3.5 cfs/acre (FIG. 5-3C)

INFILTRATION FACTOR:

SURFACES ASSUMED IMPERVIOUS FOR THESE CALCULATIONS

PEAK FLOW:

DRAINAGE AREA = 1.38 AC TOTAL EQUATION 5-6: Q(p) = A(t)q - A(p)F(i)

10 YEAR FLOW Q(p) = 1.38 (2.0) = 2.76 cfs100 YEAR FLOW Q(p) = 1.38 (3.5) = 4.83 cfs

TOTAL FLOW TO NORTHWEST (ASSUMES NO RE-CLAIM):

10 YEAR FLOW
$$Q(p) = Q1 + Q2 = 3.07 + 2.76 = 5.83$$
 cfs
100 YEAR FLOW $Q(p) = Q1 + Q2 = 5.76 + 4.83 = 10.59$ cfs

AREA 3: (EAST portion)

PROPOSED CONDITIONS: ASPHALT OR CONCRETE PAVING/LANDSCAPING L=370' n=0.2 (AVE. SMOOTH PLUS LANDSCAPE), s=6.5%, ELEVATION = 1000' T(r) = 10.66 MIN. (EQUATION 5-3)

UNIT PEAK FLOW:

T(r) = 11 MINUTES, ELEVATION = 1000'

10 YEAR FLOW q = 2.0 cfs/acre (FIG. 5-3A) 100 YEAR FLOW q = 3.6 cfs/acre (FIG. 5-3C)

PEAK FLOW:

DRAINAGE AREA = 1.33 AC TOTAL EQUATION 5-6: Q(p) = A(t)q - A(p)F(i)

10 YEAR FLOW Q(p) = 1.33 (2.0) = 2.66 cfs100 YEAR FLOW Q(p) = 1.33 (3.6) = 4.79 cfs

PEAK FLOW FROM AREA 3, POST-CONSTRUCTION, IS LESS THAN PEAK FLOW FROM AREA B, PRE-CONSTRUCTION, THEREFORE DETENTION FOR EASTERLY PORTION IS NOT REQUIRED.

DETENTION CALCULATIONS:

Q=ciA

Q from 100 year peak flow calculations = 10.59 cfs

5 min I = 5.03 in/hr (Table 5-A-3) c=Q/(i x A) or 10.59/(5.03 x 2.66) = 0.79

This is approximately the expected c value for an improved commercial site with a mix of hardscape (parking and drives) and landscaping.

Use c = 0.8

Duration (min)	Intensity*	Q (cfs)	* From Table 5-A-3, 100 year Event, 300' Elevation Placer County
0	0	0	
5	5.03	$Q=c(i \times A) = 0.80($ = 10.70	5.03 x 2.66)
10	3.57	$Q=c(i \times A) = 0.80($ = 7.60	3.57 x 2.66)
15	2.89	$Q=c(i \times A) = 0.80($ = 6.15	2.89 x 2.66)
30	1.95	$Q=c(i \times A) = 0.80($ = 4.15	1.95 x 2.66)
60	1.33	$Q=c(i \times A) = 0.80($ = 2.83	1.33 x 2.66)

SEE FOLLOWING GRAPH G1 FOR POND VOLUME REQUIREMENT.

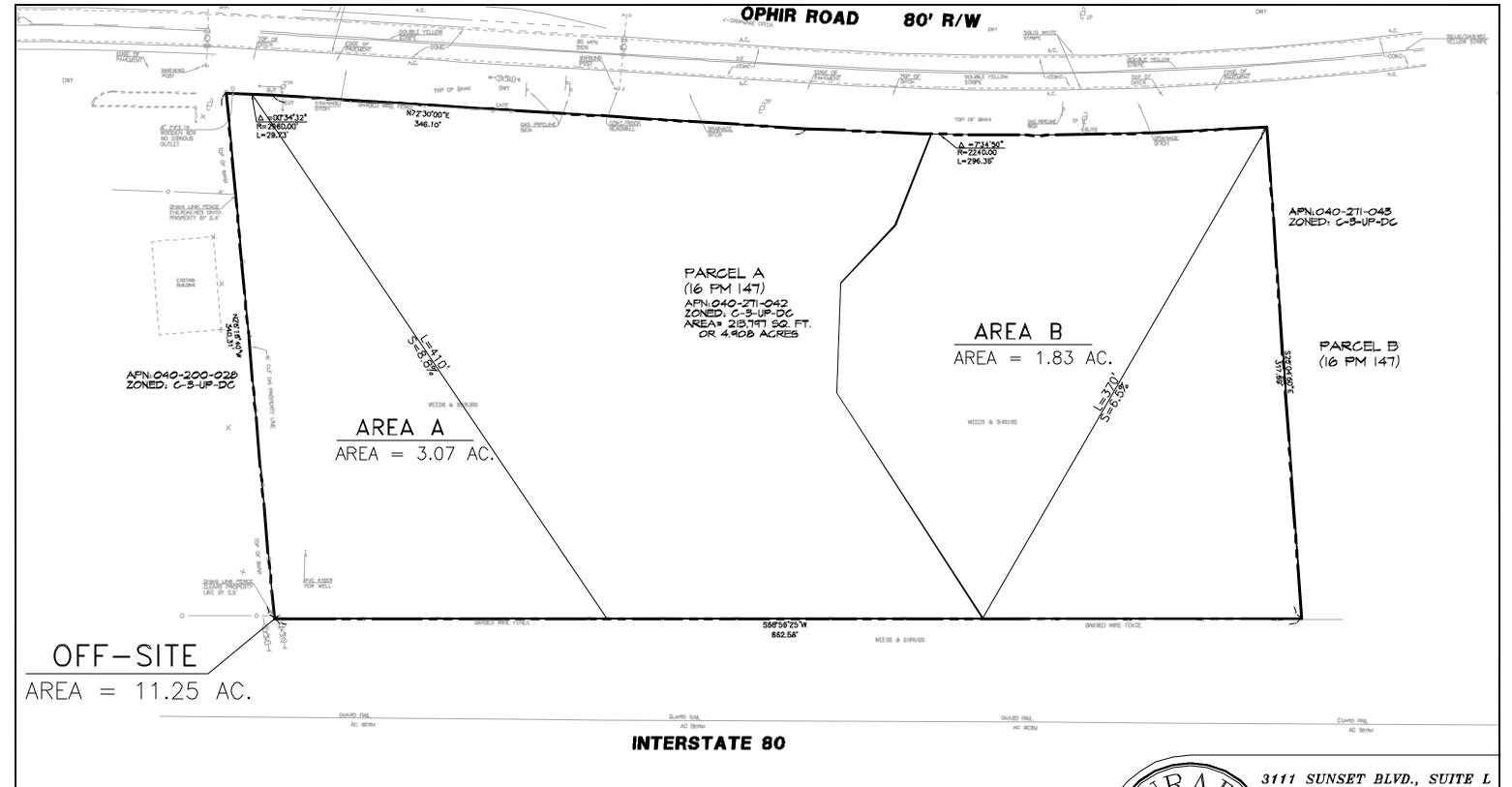


EXHIBIT 2

PRE-DEVELOPMENT

SHED MAP

SCALE: 1" = 60'

LIVINGSTON'S CONCRETE

APN: 040-271-042

OPHIR ROAD PLACER COUNTY CALIFORNIA

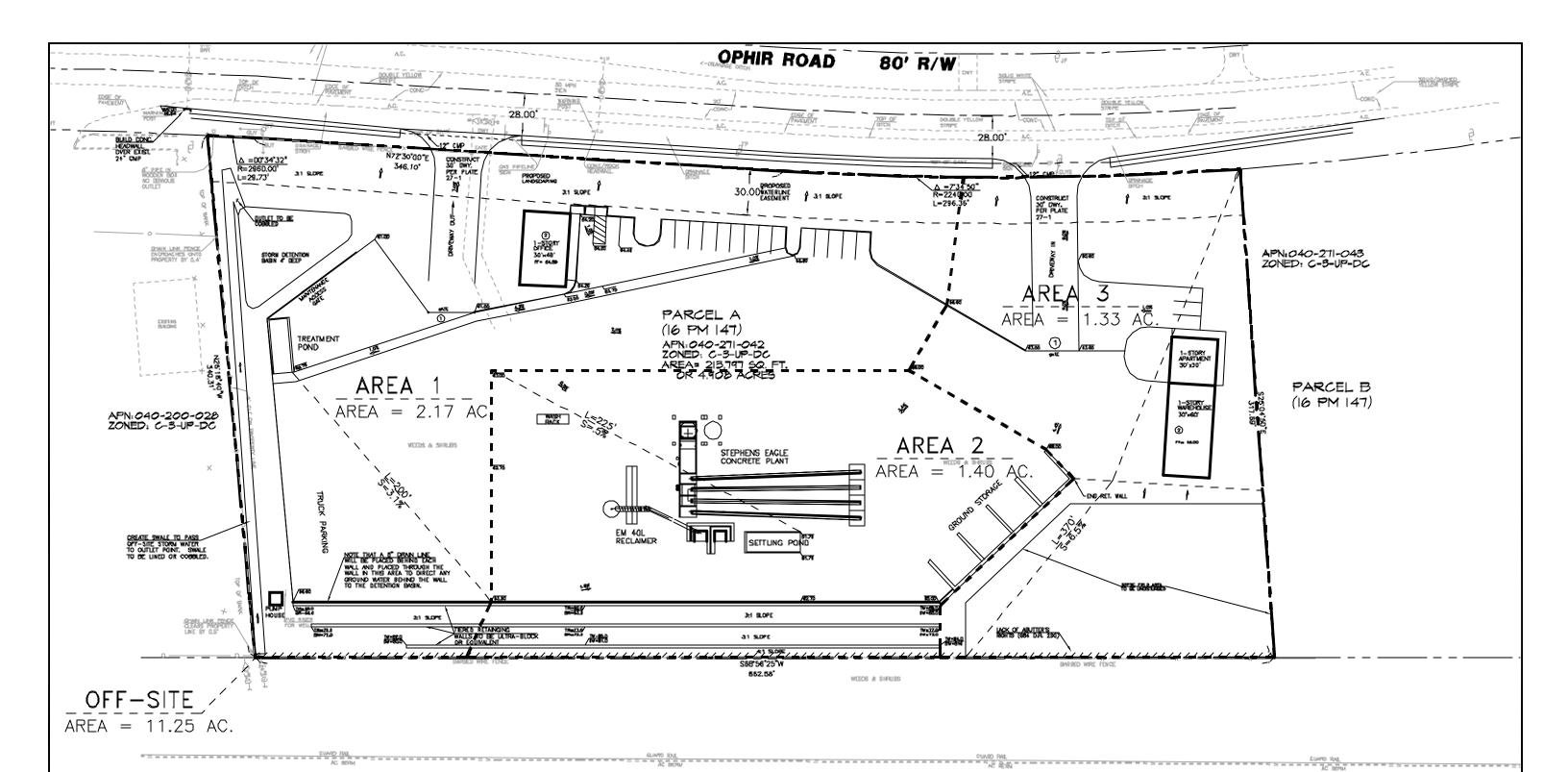


3111 SUNSET BLVD., SUITE L ROCKLIN, CA 95677 PHONE: (916) 624-1221 FAX: (916) 624-1232

ENGINEER: STEVEN OURADA

DATE: 06/22/2006

JOB NO: LC-922



INTERSTATE 80

EXHIBIT 3

POST-DEVELOPMENT

SHED MAP

SCALE: 1'' = 60'

LIVINGSTON'S CONCRETE

APN: 040-271-042

OPHIR ROAD PLACER COUNTY CALIFORNIA



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ENGINEER: STEVEN OURADA

DATE: 06/08/2006

JOB NO: LC-922



213 Sawyer St., Auburn, CA. 95603 (530) 885-0573

10 March 2006

Placer County Health Department Division of Environmental Health 11454 B Ave. Auburn, CA. 95603 Attn. Mr. Will Kirschman, R.E.H.S.

Re: On-Site Wastewater Disposal Feasibility Report- Livingston's Concrete Batch Plant

Dear Mr. Kirschman,

Based upon the results of the additional soils testing conducted by Aqua-Terra, soils in the designated disposal area were determined to be suitable for the installation of a intermittent sand filter type of wastewater disposal system. The attached report entitled On-Site Wastewater Disposal Feasibility Report Livingston's Concrete Batch Plant In Newcastle, Placer County, CA. APN: 040-271-042 provides soil testing results, design criteria and site plan which identifies the location of the wastewater disposal area.

The recommended wastewater disposal system for the proposed project would consist of a 1,200-gallon septic tank, a 1,200-gallon pump tank, a 16 ft. by 24 ft. sand filter and 180 lineal feet of disposal trench. Typical disposal trenches are to be thirty-six (36) inches wide by eighteen (18) inches deep with the flow line at approximately twelve (12) inches from the ground surface. The disposal field in the repair area will need to be capped with twelve (12) inches of native topsoil. Please note that the raw water irrigation line will need to be relocated out of the designated disposal area.

There is approximately 7,500 sq. ft. of area available for purposes of on-site wastewater disposal. In order to provide additional disposal area, it is recommended that a property line setback variance be applied for and obtained.

Final design drawings, construction details and material specifications will be prepared by the Design Consultant and submitted to Environmental Health at time of septic permit application.

If you need any additional information or should you have any questions regarding the information contained in this report please feel free to contact me a (530) 885-0573.

Sincerely,

David G. Honeycutt, M.A.

Registered Environmental Health Specialist

Cc. North Fork Associates



213 Sawyer St., Auburn, CA. 95603. (530) 885-0573

ON-SITE WASTEWATER DISPOSAL FEASIBILITY REPORT LIVINGSTON'S CONCRETE BATCH PLANT PROJECT IN NEWCASTLE, PLACER COUNTY, CALIFORNIA APN: 040-271-042

Situs Information:

APN: 040-271-042

Owner/Applicant:

Livingston's Concrete Services, Inc.

5416 Roseville Rd.

North Highlands, CA. 95660

Property Location: Ophir Rd. In Newcastle, Placer County, CA.

Parcel Size: 4.9 AC

Water Supply: Well

Introduction:

North Fork Associates is in the process of preparing environmental documents for the proposed Livingston Concrete Batch Plant project. As currently envisioned, the project will include a 1,440 square-foot office building, a 1,800 square-foot warehouse building, a concrete batch plant, truck wash and parking areas. In order to further evaluate the properties suitability for wastewater disposal, this firm has been retained to evaluate the site and satisfy the Placer County Division of Environmental Health's soil testing requirements for utilizing an individual on-site wastewater disposal system. Soil's testing was previously conduced on the subject site by David Lindbloom, R.E.H.S in October 1999.

The purpose of this report will be to present the results of soils testing conducted by this firm and provide the preliminary design specifications for the proposed wastewater disposal system.

Environmental Setting:

The subject property is currently undeveloped. Historically portions of the property have been under production as a fruit orchard. The site has gently sloping terrain with slope gradients ranging from 2 to 15 percent. The typical Flora in the area consists of Live Oak, Blue Oak, Valley Oak, Gray Pine and annual grasses. Low and wet areas also support Cottonwoods, Willows, Blackberries and Wire Grass. The Soil Survey of Placer County (U.S.D.A. Soil Conservation Service, 1980) shows the subject property to be located within the (106) Andregg coarse sandy loam, 2 to 9 percent slopes. The geology of the site as shown on the Engineering Geology Map of the Auburn U.S.G.S. Quad (June 1974) is (grt) Quartz Diorite Granitic rock unit. Surface drainage from the site is generally towards the north to northwest that drains into a tributary of Auburn Ravine

Soil Testing Procedures:

Soil profiles were evaluated by examination of backhoe test pits. Soil profiles were jointly logged with Mr. Will Kirschman, R.E.H.S., representative from the Placer County Division of Environmental Health on 21 February 2006.

This firm conducted soil percolation testing on 23 February 2006 following presoaking. The perc rate was established by taking a minimum of three 30-minute readings. The last reading was used as the design perc rate provided the difference with the prior reading did not exceed

10%. If the difference with the prior reading exceeded 10%, additional readings are taken until a stabilized rate occurred. In test holes where four or more inches of water seeped away during the first 30-minute interval the perc rate was established by taking ten-minute readings. Percolation tests were conducted according to the methods described in the Placer County On-Site Sewage Program Ordinance and Manual dated December 6, 2004.

Typical Soil Profile Description:

(Soil colors are moist unless otherwise indicated)

Soil Mantle #A: (effective soil depth @ 48 inches)

0 to 14 inches, dark brown (7.5YR4/4) sandy clay loam; very friable, subangular blocky structure; damp; slightly sticky and slightly plastic; common fine roots; smooth gradual boundary.

14 to 48 inches, dark brown (10YR4/3) sandy clay; friable to firm, subangular blocky structure; damp to moist; slightly sticky to sticky and slightly plastic to plastic; few fine roots; smooth clear boundary.

48 to 66 inches, gray, dark brown, white and black (10YR5/1, 7.5YR4/2, white and black) sandy clay, mottled; friable to firm, massive structure; saturated; no roots; sticky and plastic.

Soil Mantle #B: (effective soil depth @ 30 inches)

0 to 10 inches, dark brown (7.5YR4/4) sandy clay loam; very friable, subangular blocky structure; damp; slightly sticky and slightly plastic; common fine roots; smooth gradual boundary.

10 to 30 inches, dark brown (10YR4/3) sandy clay; friable to firm, subangular blocky structure; damp to moist; slightly sticky to sticky and slightly plastic to plastic; few fine roots; smooth clear boundary.

30 to 48 inches, gray, dark brown, white and black (10YR5/1, 7.5YR4/2, white and black) sandy clay, mottled; friable to firm, massive structure; saturated; no roots; sticky and plastic.

Soil Mantle #C: (effective soil depth @ 27 inches)

0 to 6 inches, dark brown (7.5YR4/4) sandy clay loam; very friable, subangular blocky structure; damp; slightly sticky and slightly plastic; common fine roots; smooth gradual boundary.

6 to 27 inches, dark brown (10YR4/3) sandy clay; friable to firm, subangular blocky structure; damp to moist; slightly sticky to sticky and slightly plastic to plastic; few fine roots; smooth clear boundary.

27 to 40 inches, gray, dark brown, white and black (10YR5/1, 7.5YR4/2, white and black) intensely weathered decomposed granite crushing to gravelly sandy clay loam, mottled; friable to very firm, massive structure; saturated; no roots; nonsticky and slightly plastic; gradual smooth boundary.

40 inches, refusal.

Soil Mantle #D: (effective soil depth @ 30 inches)

0 to 7 inches, dark brown (7.5YR4/4) sandy clay loam; very friable, subangular blocky structure; damp; slightly sticky and slightly plastic; common fine roots; smooth gradual boundary.

7 to 30 inches, dark brown (10YR4/3) sandy clay; friable to firm, subangular blocky structure; damp to moist; slightly sticky to sticky and slightly plastic to plastic; few fine roots; smooth clear boundary.

30 to 50 inches, gray, dark brown, white and black (10YR5/1, 7.5YR4/2, white and black) sandy clay, mottled; friable, massive structure; saturated; no roots; sticky and plastic.

Soil Percolation Test Results:							
Test Holes	P-1	P-2	P-3	P-4			
Depth (in.)	21	20	20	24			
Time (Min.)		Water Drop F	From Datum (in.)				
30(10)	3.3	0.7	3.5	1.6			
60(20)	3.1	0.6	3.1	1.5			
90(30)	3.0	0.6	3.0	1.4			
120(40)	_	-	~ -	-			
150(50)	-	-	-	-			
180(60)	-	-		-			

50.0

57.0

Findings:

Perc Rate (mpi):

Corrected Rate (mpi):

Soils in the areas tested by this firm were found to be shallow and somewhat poorly drained with effective soil depths ranging from 27 to 48 inches. Soils located in the proposed wastewater disposal area will require the use of a intermittent sand filter type of disposal system.

10.0

11.4

21.4

24.4

Soil percolation testing was conducted adjacent to the soil mantle test pits. Corrected soil percolation rates for test holes P-1 thru P-4 ranged from 11.4 to 57.0 mpi with a mean perc rate of 26.1 mpi.

Slopes in the area tested ranged from 5 to 10 percent. Refer to the attached site plan for the location of soil test pits and the area that is reserved for purposes of on-site wastewater disposal.

Wastewater Disposal System Design Calculations:

10.0

11.4

A. Proposed Use: The Livingston Concrete Batch Plant project as currently proposed will have one (1) plant operator, one (1) yard laborer and six (6) truck drivers. A one (1)-bedroom caretaker's apartment located in the warehouse building is also proposed. No public usage is anticipated. No wash water or industrial wastes are to be discharged into the wastewater disposal system.

B. Design Flow:1. Eight (8) employees at 15 gpd per person-2. Apartment-	450 gpd 120 gpd 300 gpd
C. Septic Tank Capacity	1,200 gal.
D. Pump Tank Capacity	1,200 gal.

E. Sand Filter

16 ft. by 24 ft.

F. Average Perc Rate

26.1 mpi

G. Lineal Footage of Disposal Trench

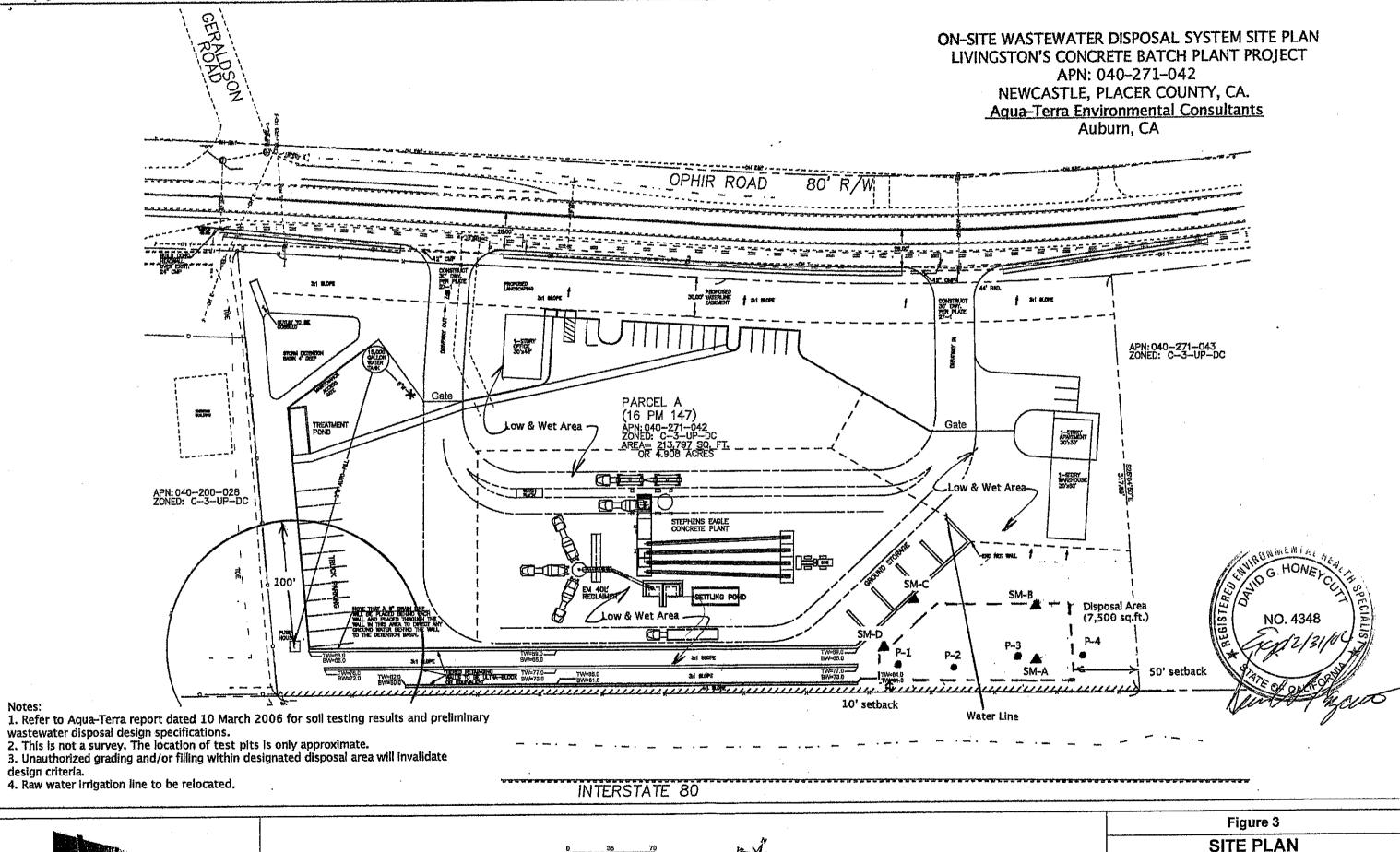
180 lineal ft.

Recommendations and Conclusions:

Soils in the designated wastewater disposal area were determined to be suitable for utilizing a intermittent sand filter type of wastewater disposal system. The recommended wastewater disposal system for the proposed project would consist of a 1,200-gallon septic tank, a 1,200-gallon pump tank, a 16 ft. by 24 ft. sand filter and 180 lineal feet of disposal trench. Typical disposal trenches are to be thirty-six (36) inches wide by eighteen (18) inches deep with the flow line at approximately twelve (12) inches from the ground surface. The disposal field in the repair area will need to be capped with twelve (12) inches of native topsoil. Please note that the raw water irrigation line will need to be relocated out of the designated disposal area.

There is approximately 7,500 sq. ft. of area available for purposes of on-site wastewater disposal. In order to provide additional disposal area, it is recommended that a property line setback variance be applied for and obtained. Contact Placer County Division of Environmental Health for details.

Prior to issuance of the building permit, the final design drawing, material specifications and construction details will need to be prepared by the Design Consultant.





Notes:





Livingstons's Concrete

Placer County, California